

Thibaut Mallet



Thibaut Mallet is a hydraulic engineer having 24 years of experience. After working in Sri Lanka and Mali from 1995 to 1998, he joined the Agricultural Ministry as a civil engineer. Following the 2002 flood in the Gard province and the Rhône flood of 2003, he became Project Director for the construction of small dams and reconstruction of levees (Aramon, Comps...). In 2006, he became Deputy General Manager of SYMADREM, a public institution in charge of the management of river and sea levees in the Rhône Delta River (240 km). He is now implementing a 400M Euros program to reinforce the Rhône levees. As part of the French regulation related to levees, he developed a model to evaluate breach probability following the guidelines of ICOLD bulletin 164. He presented his work at ICOLD annual meetings in 2014, 2016 and 2018.

Quantitative Risk Assessment for flood protection embankments using ICOLD Bulletin 164: the Symadrem experience



ICOLD Internal Erosion Workshop

Internal erosion workshop

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Presentation of SYMADREM

A public institution responsible (25 people) for :

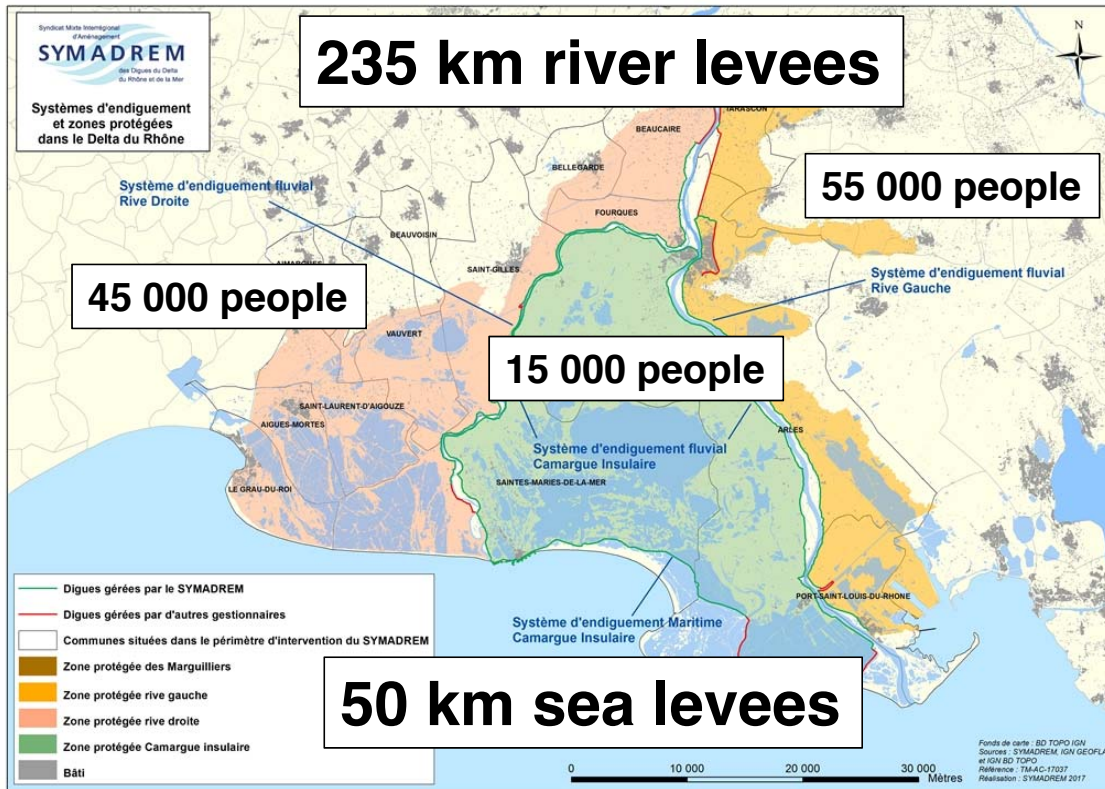
- operations and maintenance of levees in all circumstances
- levees improvement works (400 millions euros over 20 years)



$$Q_{10} = 8\,500 \text{ m}^3/\text{s}$$
$$Q_{100} = 11\,500 \text{ m}^3/\text{s}$$
$$Q_{1000} = 14\,500 \text{ m}^3/\text{s}$$



3 river levees systems and 1 sea levees system



210 km embankment levees



25 km infrastructures



30 closing gates



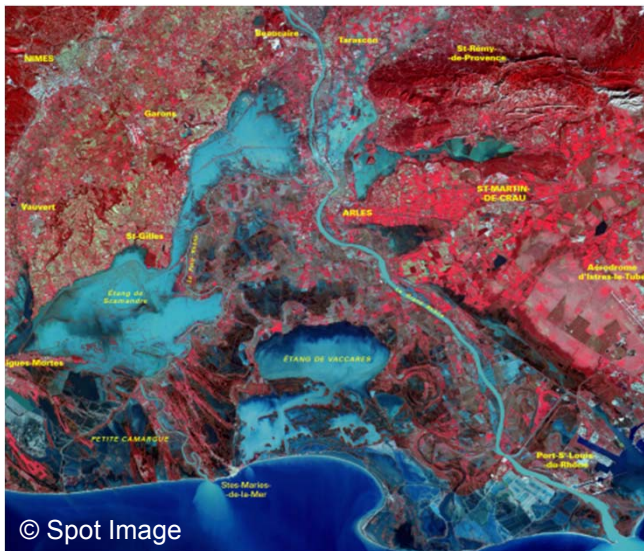
350 crossing hydraulic structures



Inundations by breaches

in 1840, 1841, 1843, 1846, 1856, 1993, 1994, 2002, 2003

December 2003 $Q = 11\,500 \text{ m}^3/\text{s}$ $T = 100 \text{ years}$



4 breaches
Spilling volume $\cong 230 \text{ million m}^3$
Cost of damages $\cong 700 \text{ million } \text{€}$

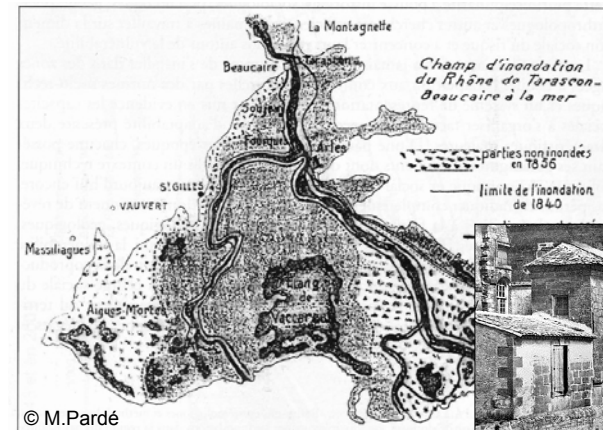


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© Arles

November 1840 & May 1856



© M.Pardé

$Q \cong 12\,500 \text{ m}^3/\text{s}$

$T \gg 100 \text{ years}$



Tarascon 1856 © E. Balbus

Spilling volume in 1840 $\cong 2800 \text{ million m}^3$

Spilling volume in 1856 $\cong 1800 \text{ million m}^3$

Estimated cost of damages today $\cong 2,8 \text{ billion } \text{€}$

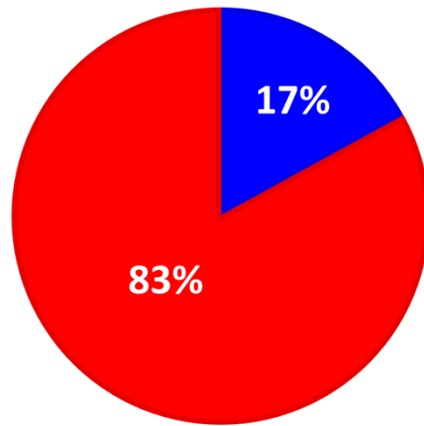
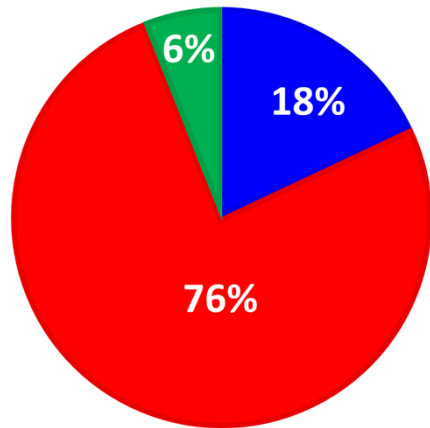


Accidentology from 1840 to today

57 breaches (with inundation) and 57 breaches in progress (no inundation)

1840-2019

1993-2019



Breaches and breaches in progress

Internal erosion => concentrated leak erosion



80 % in badger burrows



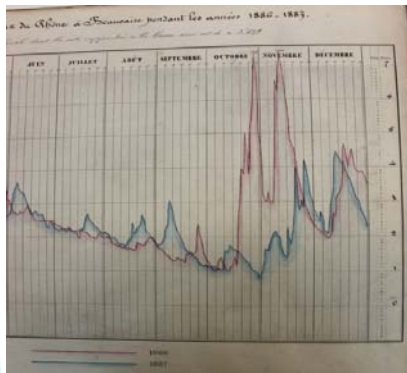
20 % along crossing pipes

■ overflowing ■ internal erosion ■ scour

Development of probabilistic model

MOTIVATIONS FOR A PROBABILISTIC APPROACH

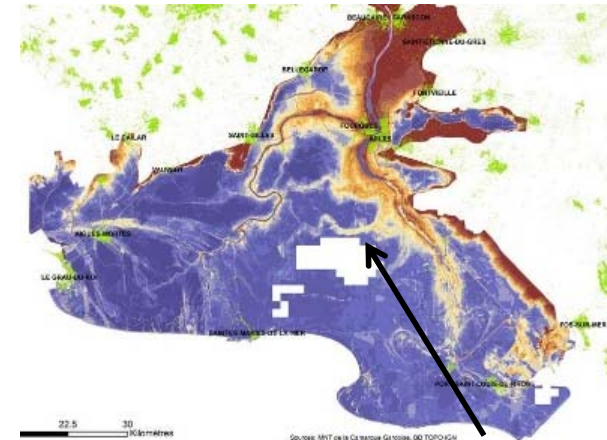
easing identification of the probabilistic nature of the hazard (data since 1816).



heterogeneous facies of the levees, due to the successive stages of their construction since the 12th century



heterogeneities of foundation due to multiple changes of the Rhône bed.

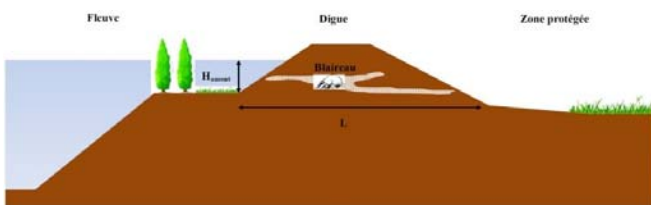


This heterogeneity makes the deterministic approach of a safety factor illusory.

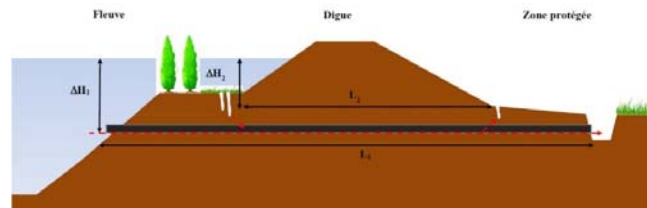
Former bed



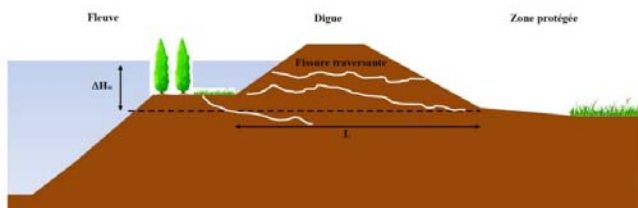
11 breach scenarios : 4 by concentrated leak erosion



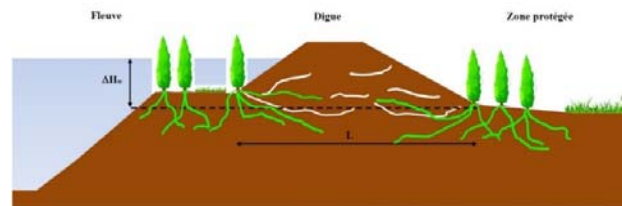
In a former badger burrow partially plugged, not visible and unknown, after hydraulic fracturing



In a hole along a crossing structure

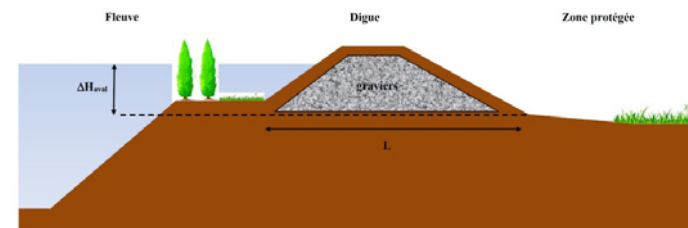
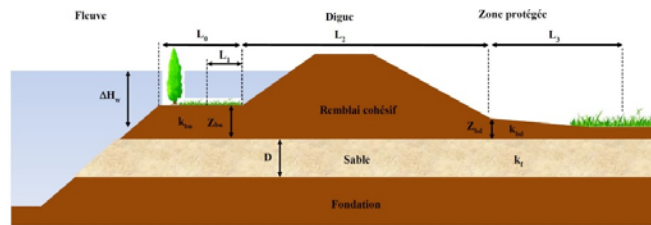


In a crossing crack along a former transition insufficiently treated



In a root of dead tree

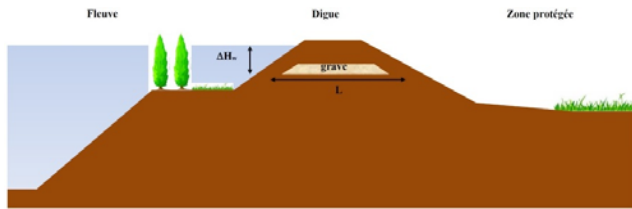
3 by other internal erosion mechanisms



Backward erosion in a sand layer

after uplift the silty blanket overlying the sandy soil strata

Contact erosion between gravel and silt (case of breaches repaired in emergency situations)

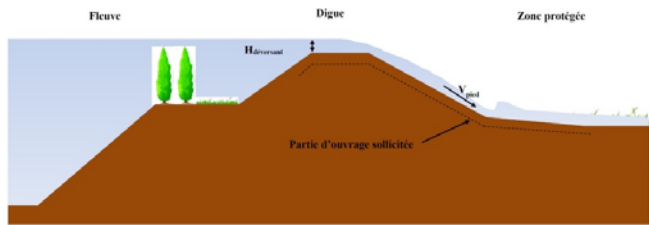


Suffusion in gravel of an ancient pavement layer

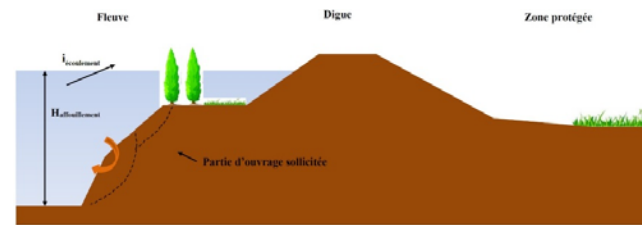
(because levees were ancient ways of communication before their general raising in the 19th century)



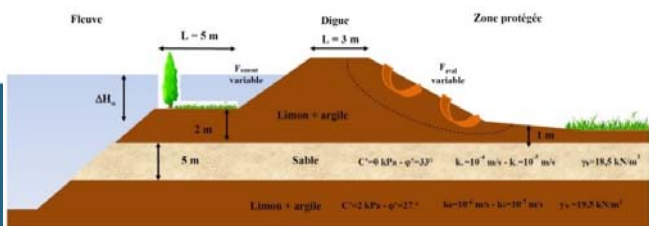
2 by external erosion and 2 by sliding



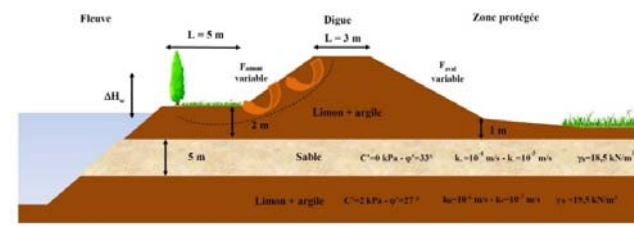
Overflowing



Scour

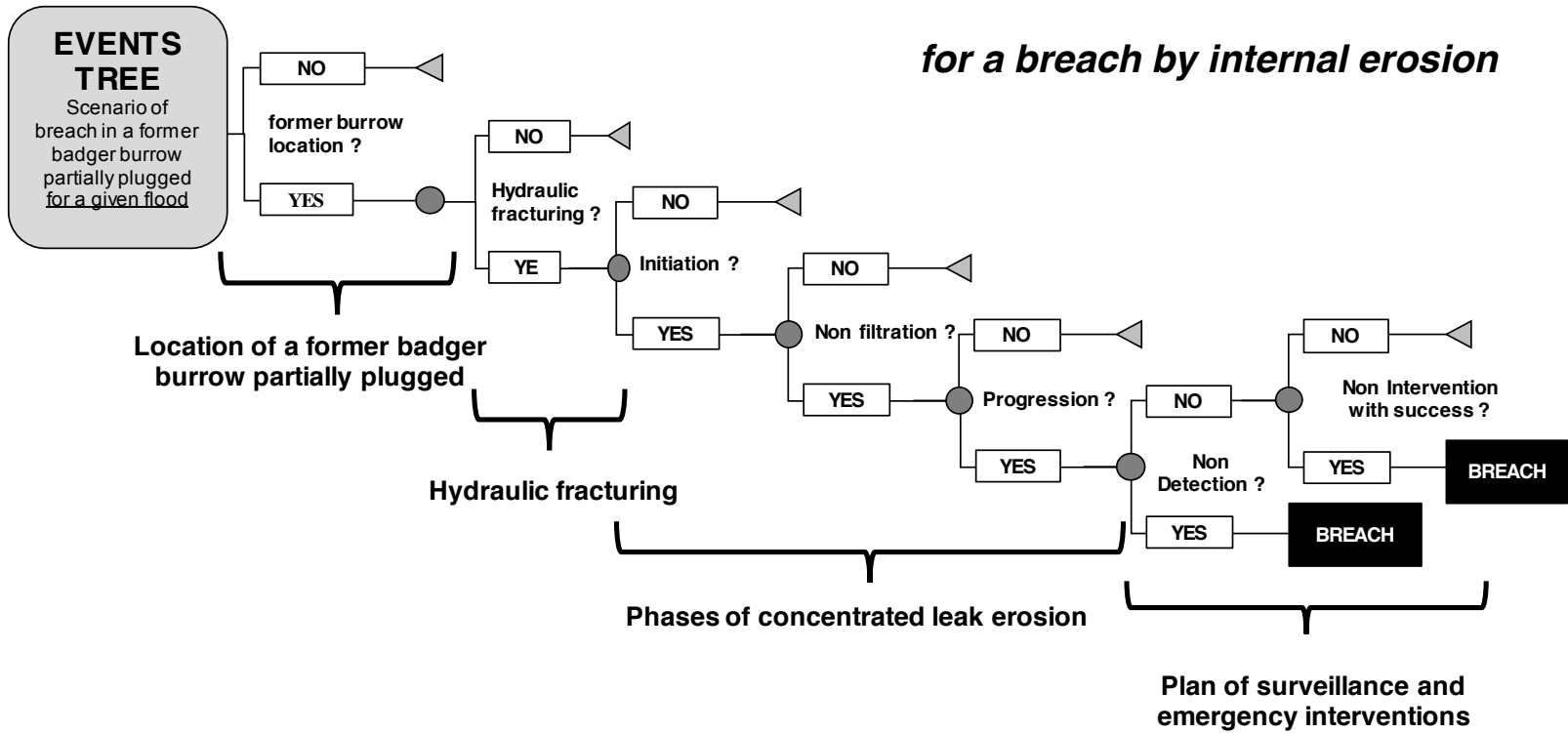
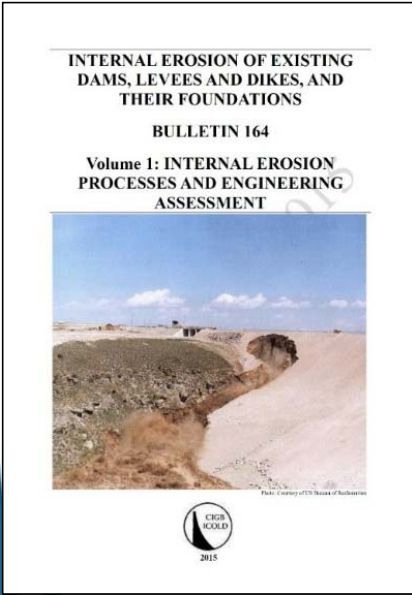


Downstream slope sliding during flood



Upstream slope sliding during flood draw down

For each breach scenario, building of a events tree



$$P(\text{breach}) = P_{\text{location}} \cdot P_{\text{fracturing}} \cdot P_{\text{initiation}} \cdot P_{\text{non filtration}} \cdot P_{\text{progression}} \cdot [P_{\text{non detection}} + P_{\text{non intervention}} - P_{\text{non detection}} \cdot P_{\text{non intervention}}]$$



Conditional probability of breach

in function of load intensity by using
subjective probabilities

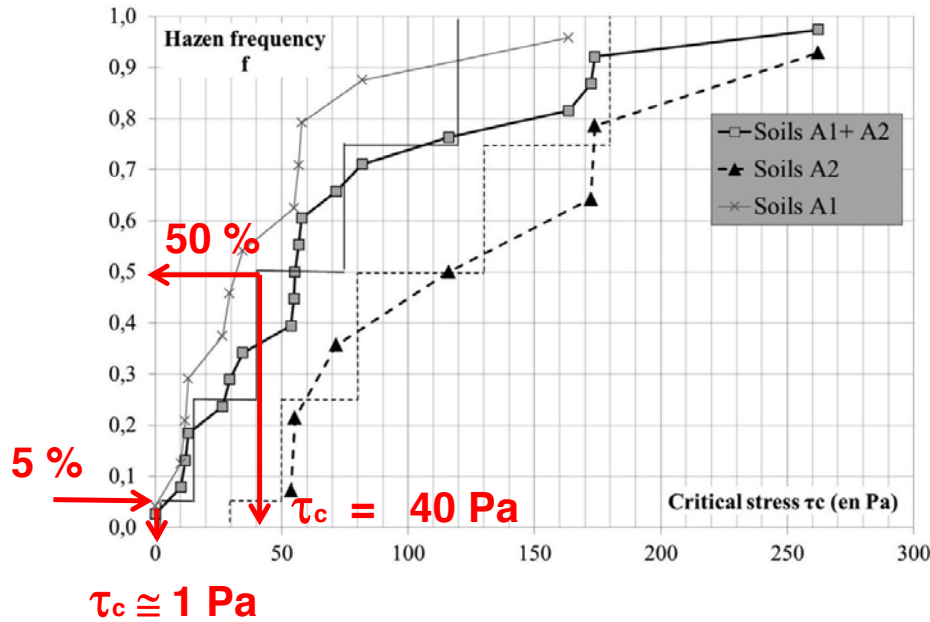
(USBR 2012 adapted from Vick 2002)

Descriptor	Assigned Probability
Virtually certain	0,999
Very Likely	0,99
Likely	0,9
Neutral	0,5
Unlikely	0,1
Very Unlikely	0,01
Virtually Impossible	0,001

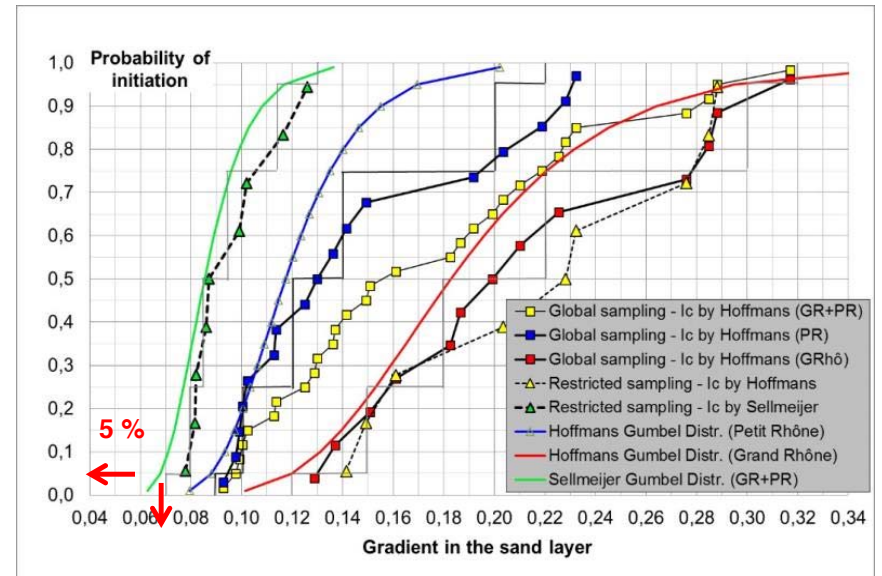
Conditional probability of breach

in function of load intensity by using **frequency probabilities**

Distribution of critical shear in coherent soils



Distribution of critical gradient in sands



$i_c = 0,07$



Probability of location (ex. former badger burrow)



Ancient levee with negative feedback
Probability = 0,5



New levee with grid against burrowing
Probability = 0,001



Probability of initiation

Concentrated Leak Erosion

Bonelli, Fell & Behnamed (2013)

initiation if $\tau > \tau_c$

$$\tau = \frac{1}{2} \cdot R \cdot \alpha \cdot \rho_w \cdot g \cdot i \quad \alpha = \left(1 + \frac{kR}{4Lf}\right)^{-1}$$

Backward erosion

Sellmeijer (2011)

initiation if $i > ic$

$$ic = \frac{\gamma'_p}{\gamma_w} \tan \vartheta \left(\frac{RD}{RD_m}\right)^{0.35} \left(\frac{U}{U_m}\right)^{0.13} \left(\frac{KAS}{KAS_m}\right)^{-0.02} \frac{d_{70}}{\sqrt[3]{\kappa L}} \left(\frac{d_{70m}}{d_{70}}\right)^{0.6} 0.91 \left(\frac{D}{L}\right)^{\frac{0.28}{2.8} - 1} + 0.04$$

Contact Erosion

Beguin(2011)

initiation if $V > V_c$ $V = k \cdot i = 7 \cdot 10^{-2} \cdot i$

Darcy Velocity	Probability of initiation	Comments
1 cm/s	0.01	No érosion
1,5 cm/s	0.5	
2 cm/s	0.9	Erosion

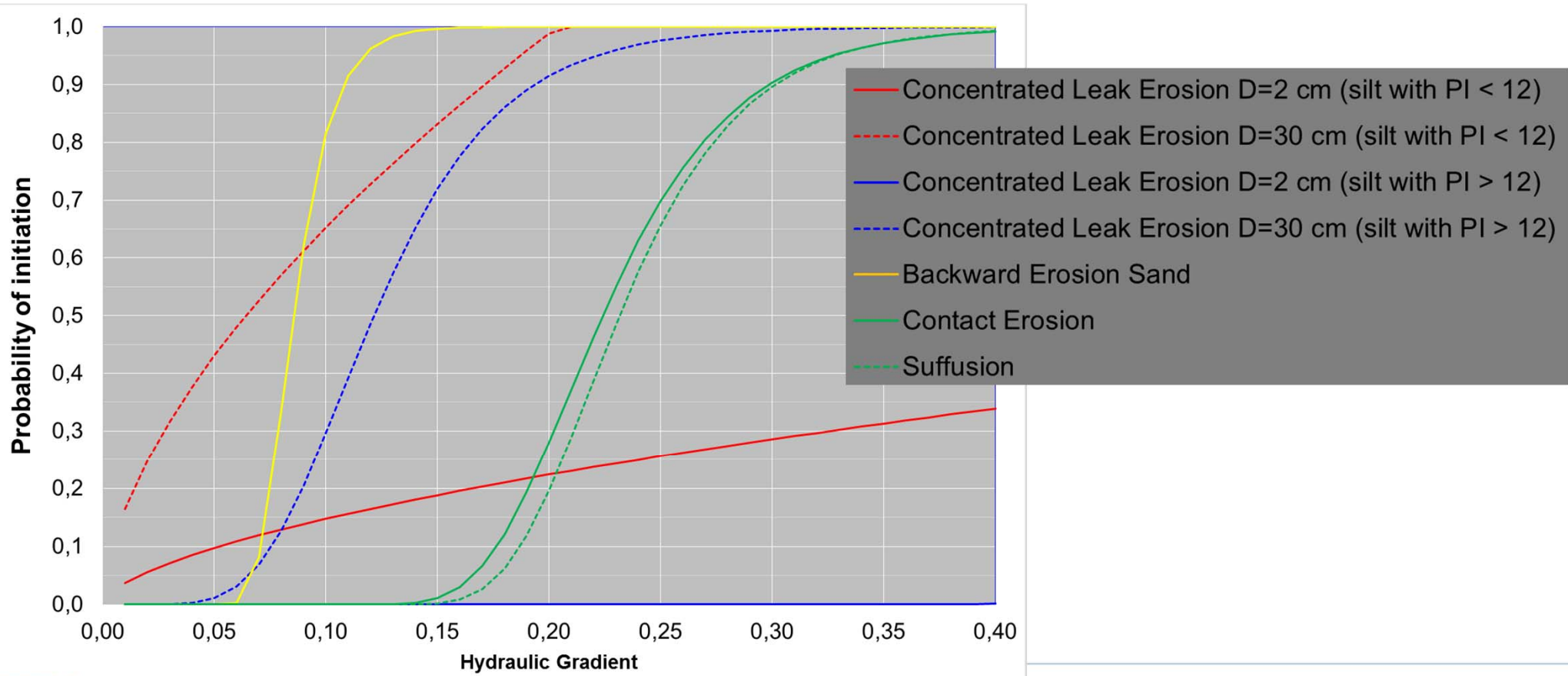
Suffusion

Wan & Fell (2004, 2007); Marot&al (2012)

Hydraulic Gradient	Probability of initiation	Verbal Qualification verbale
0,1	0.001	Virtually impossible
0,15	0.01	Very unlikely
0,2	0.1	Unlikely
0,3	0.9	likely



Probability of initiation by internal erosion mode





Probability of non filtration



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No filter
Probability = 1



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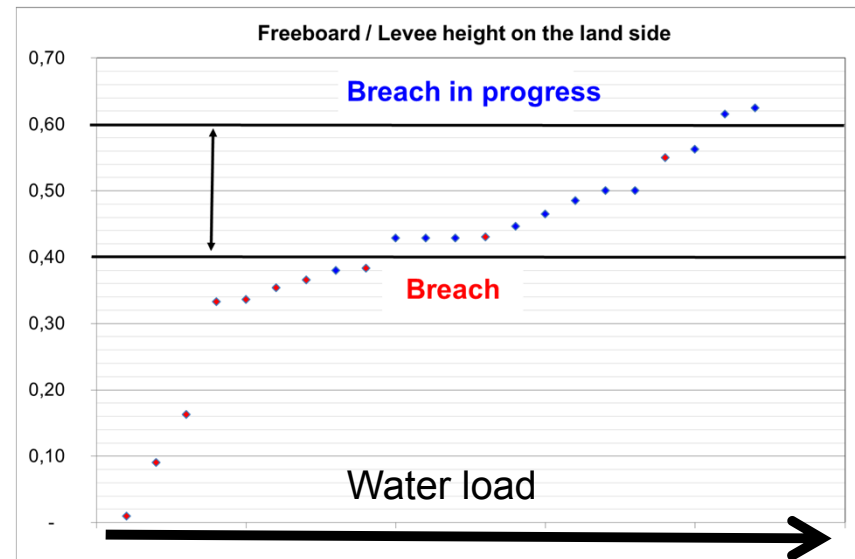
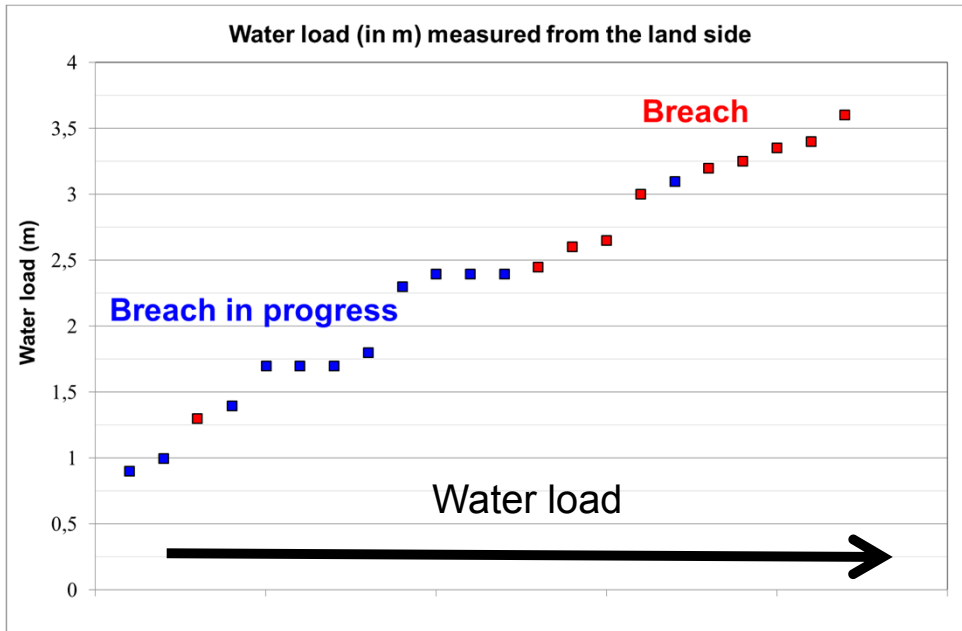
Filter but uncertainties
Probability = 0,1



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Filter well designed
and constructed
Probability = 0,01

Probability of progression





Probability of non detection and non intervention

$$P_{non\ detection} = \max(0,01; \left(1 - \frac{\Delta t_u}{P_s}\right))$$

$$P_{non\ intervention} = \min(1; \frac{\Delta t_i}{\Delta t_u})$$

P_s : duration between 2 visits (between 3 & 6 hours) checked during flood in november 2016

Δt_u : time from detection to failure

Δt_i : intervention time (trafficable crest => 3 hours or not => 24 h) => checked in 2016 and 1993

For concentrated leak erosion

$$\Delta t_u \approx \frac{2\rho_d}{C_e \alpha \rho_w g i} \ln\left(\frac{R_u}{R_d}\right)$$

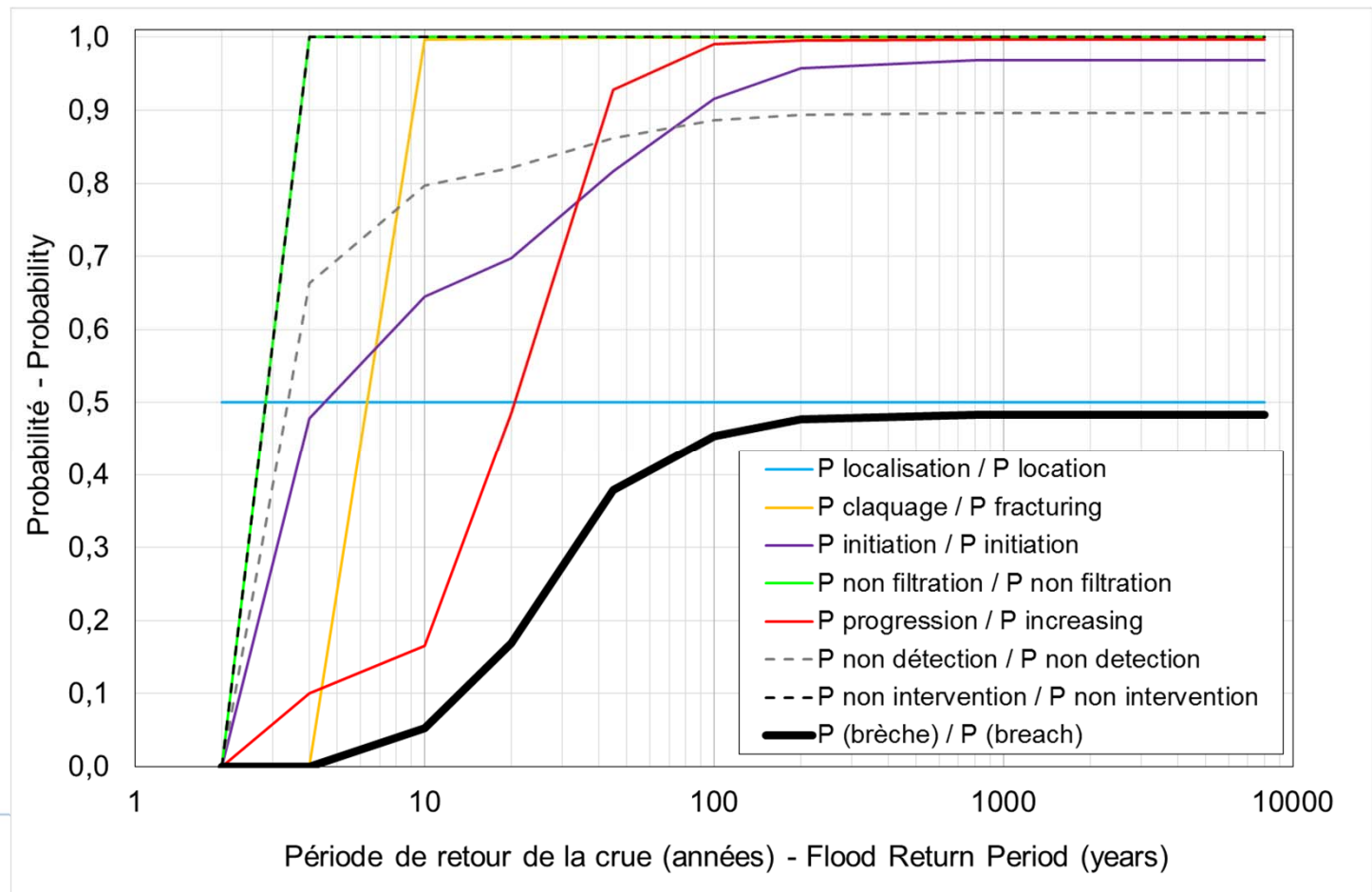
For backward erosion $\Delta t_u \approx 24h$

For contact erosion and suffusion $\Delta t_u = 48h$



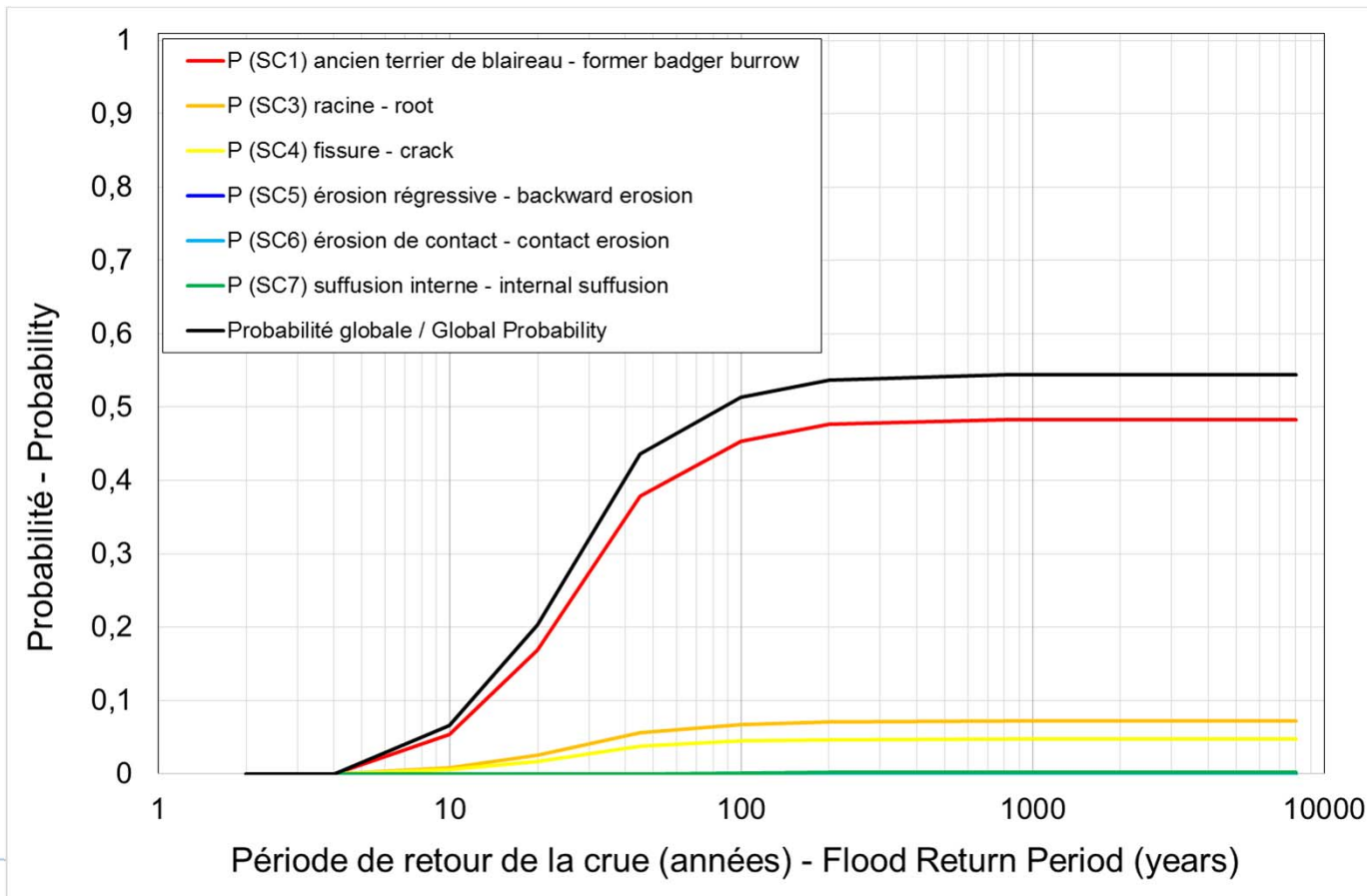


Conditional probability of each scenario





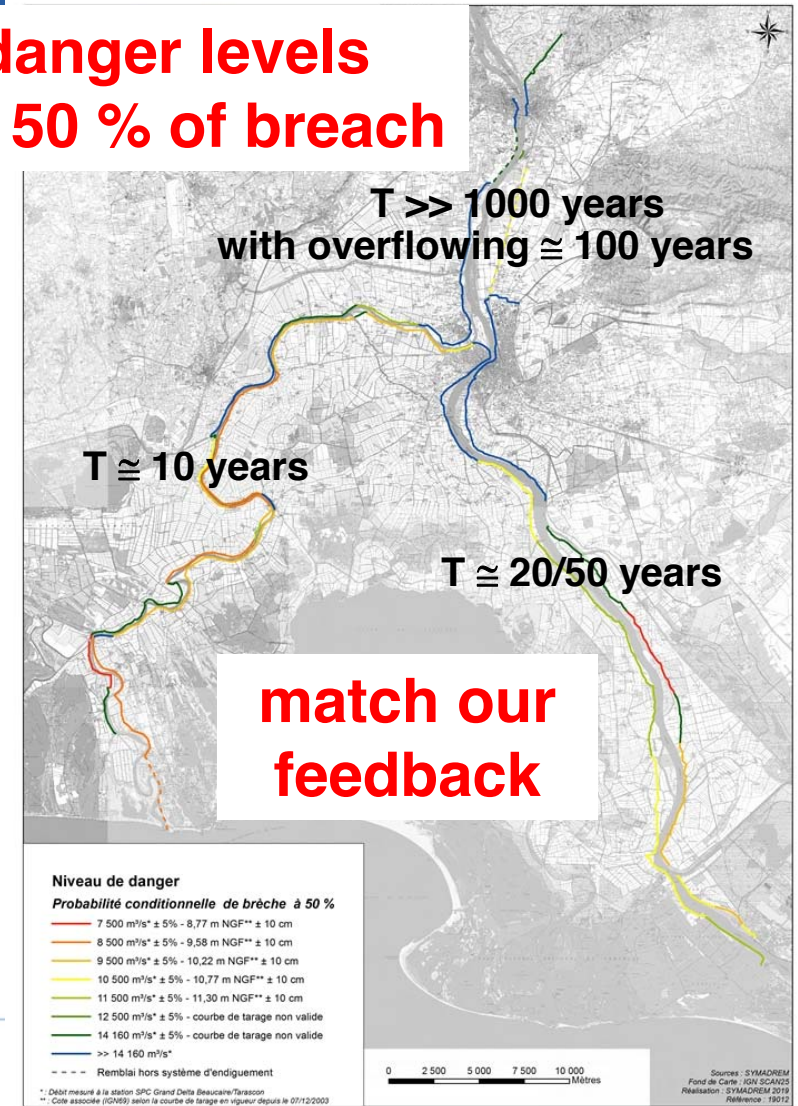
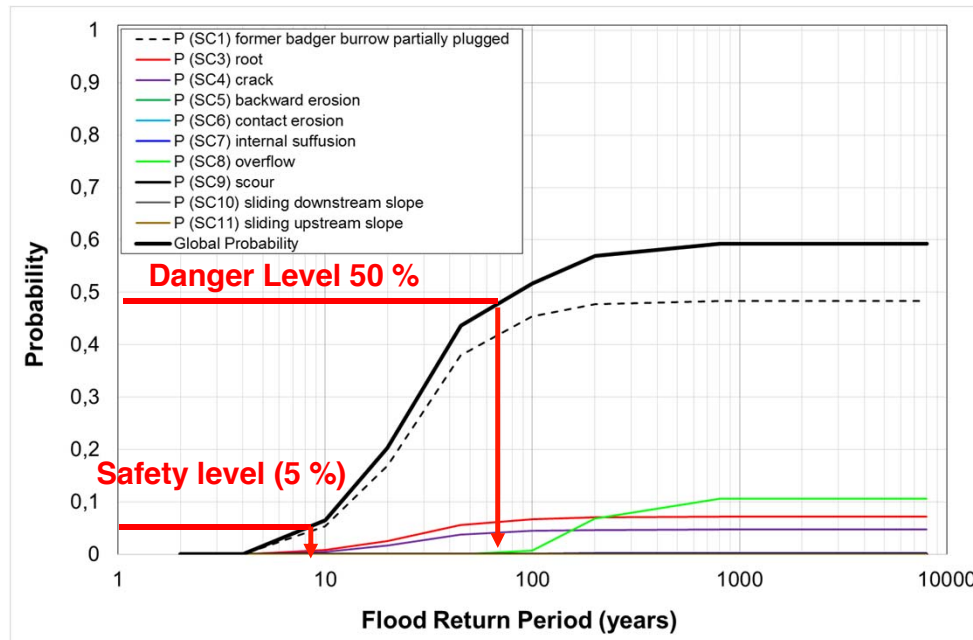
Global probability of all internal erosion mechanisms





Conclusion

**danger levels
P = 50 % of breach**

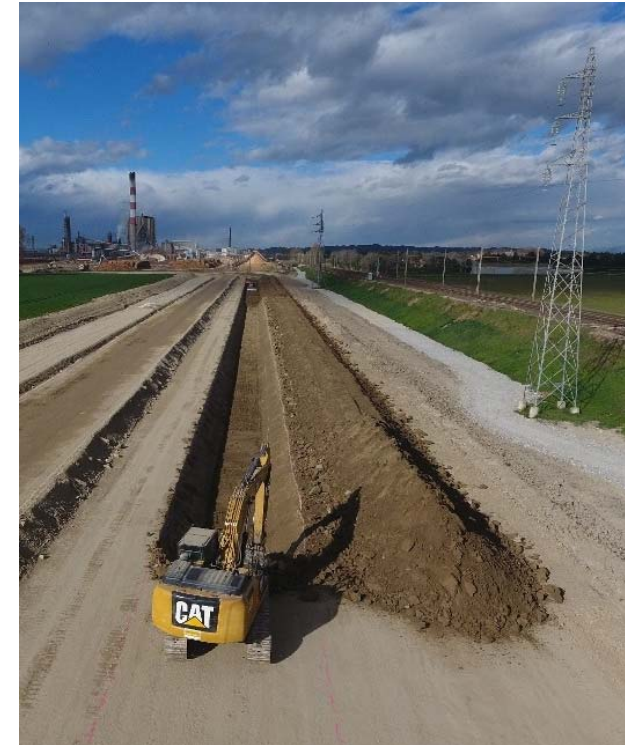


more details at <http://www.symadrem.fr/enquete-publique/>

pages 1280 to 1510



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