

Downstream Spillway Erosion in EDP Dams

Irene Ramos Fernandes

José Dias da Silva

EDP Produção Dam Engineering Division



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- **EDP Hydropower Plants**
- EDP Dams in Portugal
- Downstream Spillway Erosion Problems and Corrective Measures
 - Caniçada and Salamonde Dams
 - 🗸 Santa Luzia Dam
 - Picote Dam
 - Paradela Dam
- Recent dams Adopted Solution
- Final Remarks



EDP Hydropower Plants



EDP Hydropower Plants in Portugal

Hydropower plants (>10MW)



EDP Produção is responsible for the operation of 82 hidropower plants, with a total installed capacity of 6944 MW.

Around 2599 MW are installed in reversible units, allowing the operation in turbine and pump modes.

The power plants are organized in 3 Regional Generation Centers, associated with the respective river basins:

- Cávado-Lima (
- Douro
- Tejo-Mondego (3)

Small hydro (≤10MW)



EDP Hydropower Plants

EDP Dams in Portugal

Downstream Spillway Erosion – Problems and Corrective Measures

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EDP Dams in Portugal



- Around 50 large dams
- 12 dams in international rivers
- Oldest dam completed in 1927
- Most recent completed in 2017
- Wide range of heights (max. 132 m)
- All structural types, with the exception of earthfill
- All dams in concrete, except 3 (rockfill)
- All dams founded in bedrock, except 1 (partly in alluvium)



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Caniçada and Salamonde Dams





Caniçada Dam - Original Situation



- Year of completion: 1955
- Dam type: Thin arch
- Height: 76 m
- Foundation: Granite
- Spillway type: Controlled; free falling jet
- Maximum discharge capacity: 1700 m³/s





Cross section

Caniçada dam – Repair Works in 1960-61



Weir: 115 m downstream the dam



Caniçada Dam – After the Execution of the Downstream Weir



Caniçada Dam – Downstream River Bed



2017

Caniçada and Salamonde Dams



Caniçada dam

1700 m³/s + 2000 m³/s

Salamonde dam

1700 m³/s + 1000 m³/s



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Santa Luzia Dam







Upstream view

- Year of completion: 1942
- Dam type: Thin arch
- Height: 76 m
- Foundation: Quartzite
- Spillway type: Controlled; free falling jet
- Maximum discharge capacity: 120 m³/s



Santa Luzia Dam – Downstream Spillway Erosion



- Until 1997: scour depth 3m
- Between 1998 and 2001: scour depth 7m





Santa Luzia Dam – After Repair Works in 2003





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Picote Dam



Douro – Portuguese Hydropower schemes



Picote Dam



- Year of completion: 1958
- Dam type: Arch
- Height: 100 m
- Foundation: Granite
- Spillway type: Controlled; sky jump
- Maximum discharge capacity: 11 000 m³/s

eda



Cross section

Picote Dam – Before Repowering (2007)





Longitudinal profile of the river

Picote Dam – Repowering (2007-2011)



Picote Dam – Repowering (2007-2011)





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Paradela Dam

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Paradela Dam – Original Situation



Paradela Dam – Original Situation



Paradela Dam – Original Situation

GATED SPILLWAY OUTLET

DISCHARGES THROUGH GATED SPILLWAY

PROTOTYPE TEST PERFORMED IN 1978

Discharges caused:

- Strong erosion process along the whole reach of Sela stream, downstream of the spillway outlet
- Instability of large stretches of its banks, mainly the right one, with incipient slidings and possibility of larger ones

The exploitation of the Paradela reservoir was conditioned in order to guarantee a reserve volume for flood routing, and avoid or minimize the need for the gated spillway operation.

River

Cávad

Paradela Dam – New Spillway





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Recent Dams – Foz Tua and Baixo Sabor



- Year of completion: 2017
- Dam type: Arch
- Height: 108 m
- Foundation: Granite
- Spillway type: Controlled; free falling jet
- Maximum discharge capacity: 5500 m³/s



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Final Remarks



- Systematic river bed observation and survey data analysis have been key to safety control procedures, allowing for the effective management of situations in which actual behavior deviates from expectactions.
- A multidisciplinary approach, involving the areas of hydraulics, geology, geotechnics and structures, has allowed the definition of corrective measures.
- Hydraulic model tests, performed by the National Laboratory for Civil Engineering, have been essential for the definition of suitable solutions.
- In recente dams, despite careful studies, design and construction, what kind of erosion problems can be expected in the impact stilling basin? Would it be possible to optimize this type of solution? Is there any experience of pressure and velocity measurements in prototypes?
- The challenge is to define and implement the most suitable solutions regarding efficiency and economy.





Thank you for your attention